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Septic Systems and Treatment Plants / Service and Maintenance Inspections and Assessments / Permits and Design

May 19, 2020

File #1926-19-20

Comox Valley Regional District Building Services 600 Comox Road Courtenay, BC V9N 3P6

RE: Letter of Assurance –CVRD Sewage Compliance for Building Permit Royston Elementary School, 3830 Warren Avenue, Royston, BC VOR 2V0

I, Tobin Laughlin, assure that I have completed all required reviews and inspections for the assessment of the existing sewage disposal system.

This letter assures that these inspections and reports meet all the required assessments as per the SSSPM Ver. 3 under Section II-2.1.4 and III-2.1.4.

As it applies to the above noted property and its sewage disposal system: "The system is not malfunctioning and has been appropriately constructed and installed given the size and projected demands on the system to be placed by any building or improvements that are to be served by the system."

Yours truly, ROCK CREEK ENVIRONMENTAL Tobin Laughlin BA, ROWP (BC) Reg. #OW0007



c.c. Ian Heselgrave Director of Operations School District 71 (Comox Valley) Phone: 250 334-5516 E-mail: <u>ian.heselgrave@sd71.bc.ca</u>



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May 19, 2020

File #1926-19-20

Ian Heselgrave Director of Operations School District 71 (Comox Valley) Phone: 250 334-5516 E-mail: <u>ian.heselgrave@sd71.bc.ca</u>

Compliance Inspection of the Onsite Sewage Treatment System serving Royston Elementary School, 3830 Warren Avenue, Royston, BC VOR 2V0

Type of Inspection

At your request, I attended this property to carry out a compliance inspection of the onsite sewage treatment system serving the school with the aim of determining its location, condition, operation and suitability for your needs and to support a building permit for the addition of 2 modular classrooms. The intention of adding this portable is to create room for an increased enrollment of 50 students. The current student enrolment has 267 students and the additional students will increase the schools total student enrollment population to approximately 320.

The inspection will address the current system and review all available documentation provided by the school district, building conytractors and VIHA. The inspection will include inspecting all the tanks, pumps and dispersal fields at site. This report will discuss the function, currant condition any required setbacks/environmental impacts.

This inspection meets or exceeds the industry standard as set out in the Standard Practice Guidelines for the Inspection of Onsite Wastewater Systems published by the Applied Science Technologists and Technicians of BC (ASTTBC). As a Registered Onsite Wastewater Practitioner through ASTTBC, an Inspector is required to meet or exceed these requirements and undertake this work in the best interests of the client at all times.

There will be no increase in the daily design flows from the original permit to the existing system. The property is connected to municipal water.

1.0 Objectives

The purpose of my site reconnaissance was to conduct a Sewage System Compliance Report and analysis of system at site. This report will determine the new proposed daily design flows, its effect on the existing system, ensure proper setback distances and Sewage System function of all components. Specific objectives were as follows:

- 1. Advise on the probability of a health hazard resulting from the newy classroom being connected to the existing system at site.
- 2. Ensure correct and allowable horizontal distance from components of the sewerage system to the Modular Classrooms.
- 3. Provide report that confirms the setback distances.
- 4. Provide report on system type, condition and its function of all existing components.
- 5. Provide report on the operations and on maintenance measures to reduce a health hazard.
- 6. Provide comment on impact of new total number of full time students affect on the Daily Design Flow original system.
- 7. Ensuring the system will not create a Public Health Hazard when operated and properly maintained.
- 8. Ensure the existing system conforms to the SSR and acceptable construction practices.

1.1 Limitations

This report simply approves the propsed new daily design flow, both vertical and horizontal separations. Critical setback distances from the sewage system will also be addressed. Tobin Laughlin will not be responsible for any installation of materials or workmanship of any parts of the sewage system at site. The School District will be responsible for all planned operatiuons and maintenance. The operations and maintenace should follow the currant SSR and SPM3. The BC Sewerage System Regulation is to be adhered to, as such, he will be responsible for the report, setback drawings, specifications and field reviews of the building placement and sewage componants.

This letter is subject to the attached Statement of General Conditions (Appendix 1).

1.2 System Records:

To assist with assessing the system capabilities and performance requirements, either a copy of the following records were provided or we obtained them. See Appendix 3.

- Sewerage System Permit issued date: Jan15,1996, as a repair.
- Designed For: Total DDF of 22.5c/m. under the BC,SSR Reg 411/85 oc. "innovattive Systems".
- The Permit to Construct dated: April 18/96, Condition of 300 students "while system is under Innovated Review".
- Authorization to Use: #387/94 May 30/96, Conditions as per service agreement & weekly logs.
- As-Built Drawings: Yes
- Operation & Maintenance Plan: Yes
- The site is served by municipal water System. There are no known potable wells within 30m.

NOTE: For all VIHA and system records see Appendix 3.

1.3 Type of Sewage System Present:

This system type was installed under the Innovated Systems or "Code of Good Practices" of the Health Act's Sewage System Regulation, REG 411/85, Section 3.0 and Schedule 6 (See Appendix 2).

This property has a single onsite sewage system, which consists of two 3000 ig septic tanks with an effluent pump in second chamber of second tank, a Hydroxyl Systems Inc HSI-20 treatment plant and a pressurized dispersal field of 8 laterals of approximately 18.2m each. A Total Of 122m

The dispersal field is a subsurface dispersal system using pressurized laterals covered by gravelless chambers at shallow depth.

There are 3 environmental monitoring wells (piezometer) for groundwater sampling.

The supplied Health Authority records show that the system was designed to serve a Total Daily Design Flow (DDF) of 22,500lpd or 4950igpd. The calculation for system design was used following the SSR 411/85 Schedule 2, Appendix 1 (Schedule 2, Section 14) [BC Reg 199/86, S.8.] See Appendix 3.

1.4 Evaluation of the System's Condition Septic Tank:

The first components of the sewage system are the septic tanks, located at the East (Livingston Road) side of the property. (See Drawing).

Septic Tank #1

- The top of the tank access lids are located at grade.
- The two chamber concrete tank has a capacity of 3000ig, which matches the sewerage system records.
- There is cleanout port lid located over the inlet pipe. From this point, all wastewater flows enter into the first tank's first chamber.
- The second chamber assists with settlement of the waste.
- The second chamber has an outlet inspection port and outlet pipe baffle.
- This tank flows by gravity into the tank #2.
- This tank and its chambers are as per the manufacturer's specifications.
- All lids are secure.

Tank #2

- The top of the tank access lids are located at grade.
- The concrete tank has a capacity of 3000 ig, which matches the sewerage system records.
- There is cleanout port lid located over the inlet pipe. All wastewater flows from tank #1 (the collection tank) and enters into the chamber of tank #2 for additional settlement.
- The second chamber of tank #2 is the pump chamber that transfers the effluent from the first two tanks to the "Hydroxyl Sewage Treatment Unit" model# HSI-20 for final treatment.

NOTE: All buildings, structures and utilities must remain great than 1.0m from all tanks.

Hydroxyl Systems Inc. HSI-20:

The Hydroxyl sewage treatment system process is primarily a fixed film wastewater treatment process that is designed for municipal, industrial and small flow wastewater treatment applications that need to meet NSF/ BNR discharge requirements.

The hydroxyl sewage treatment system has many treatment designs. The primary design process uses Media Retention Screen, using stainless steel wedge wire screens that then retain the cultivated biofilm/media in a process-designated reactor while allowing the treated wastewater and sloughed biofilm to grow through to the next treatment phase.

The system is basically a Single-Pass biological process. The technology employs thousands of polyethylene biofilm carriers or other suspended constructed media, operating in a mixed motion within an aerated wastewater treatment basin/tank. Each individual biocarrier increases productivity through providing and protecting the surface area of the suspended media/biocarriers by supporting the growth of heterotrophic and autotrophic bacteria within its media/cells. It is this high-density population of bacteria that is required to achieve the high-rate biodegradation within the system. This process is basically self-maintained with an optimum level of productive biofilm. The biofilm will attach to the mobile suspended media/biocarriers within the system automatically responding to load fluctuations. However, the process requires proper maintenance of the collection tanks and the raw sewage influent to hydraulic flow ratio to ensure proper function. The system's aeration cell/tank with the suspended media/biocarriers are the system's primary treatment functions. With influent loading or hydraulic flows surpassing the system design of the aeration cell of media/biocarrier surface area the system will stop producing the designed effluent qualities required.

Dispersal Field:

The dispersal field is located at the rear of the property on the East property line.

- The field is a Constructed Bed of 8 laterals
- Each Lateral is 15.2m in length.
- Each lateral is constructed of 1" PVC pipe with 7/16 drill orifices
- Each lateral is covered in a gravelless "Infiltrator H-10 STD" chamber
- Each of the laterals has a ball valve and end cap clean-out.
- There is an up-slope interceptor drain as per design.
- The system appears to be constructed as per the Permit to Construct #387/94 May 30/96.

NOTE: All buildings, structures must remain great than 3.0m and 7.5m if down slope, from dispersal field.

All utilities must remain greater than 1.0m from dispersal field.

2.0 Soil Testing

Soils were reviewed on May 2, 2020 by Tobin Laughlin of Rock Creek Environmental. I reviewed four test pits on the same date just outside the existing dispersal field area. Four 80cm deep test pits were hand dug on site (See Figure 1). Soil characteristics have been confirmed to be a few inches of topsoil and turf, over a layer of tan loamy fine sand and sandy loams and compacted till of low permeability.

2.1. Summary of Site Evaluation

On May 2, 2020 Mr. Laughlin tested evaluated site and soil conditions. Following site visits were included for well locations. The following table is a summary of the site evaluation.

able 3: Site Evaluati	on Summary					
Land Slope:	5-7% averag	е.				
# of soil test sites:	4					
Typical soil at infi	Itration surface	e where Tested				
Texture:	Sandy Loams					
Structure:		ingle-grain moderate grade slight grainess				
Consistence:	Firm	irm				
Typical soil profile						
A: Brown/tan Sand	y loam SG-0 to	GR-2, loose.				
B: Olive sand.						
C:Till, modled						
Typical depths in		spersal area				
Roots:	46cm- 62cm					
Mottling:	few distinct mo					
		e upslope drainage.				
Reference:	WL measured L	by TL				
Soil Permeability						
# of CHBP:	4	constant head borehole permeameter				
Shallow Soils						
Measured K(fs):	mm/d <i>median</i>	value, shallow soil depth of 20 to 30 cm				
-or, in mm/d	>4400mm/d	design value based on median				
<u>Deeper Soils</u>						
Measured k(fs):	2460mm/d	median value, soil depth of 45 to 50cm				
Interpretation						
FRH:	>80cm Flow Re	estrictive Horizon				
SHWT:	>50m Season	al High Water Table				

Table 3: Site Evaluation Summary

2.2 Soil Permeability

Permeability tests were conducted within the locations shown near the test pits (see Fig 1). The permeabilities KFS ranged between 4800mm/day-3800mm/day amongst the areas tested.

of CHBP tests: 4 constant head borehole permeameter

SHALLOW SOILS

Measured K(fs): cm/d median value, shallow soil depth of 20 to 30 cm. -or,inmm/d: 440 cm/d Design value based on median

NOTE: following SPM3 Table II-22, the maximum allowable Hydraulic Loading Rate (HLR) for this type of soil classification is $70L/D/m^2$. When using the maximum allowable HLR based on permeablity the HLR would be $80L/D/m^2$.

2.3 Groundwater

High groundwater conditions were not a concern during the initial test pit reviews. Checking the groundwater level on May 2, 20120 confirmed groundwater levels to be low. The upslope drain appaers to be working correctly reducing ground water

3.0 Regarding the Proposed Construction:

The proposed location of the two new modular classroom will not impact either the sewage treatment facility or its discharge and dispersal field.

The additional new modular classrooms are required for an additional 50 students. The modular classroom will be located with a setback distance of greater than 1.0m of the collection tanks and greater than 3m from the dispersal field (See Fig 1). This proposed placement meets the current Sewage System Standards Practices Manual Ver.3 for building setback to a septic tank or holding tank. It is also understood that no structures are to be placed on or near the dispersal field and or its components within 7.5m.

The additional students being added to the daily design flow will increase, the sewage system's components (collection tanks, sewage treatment plant and the dispersal field) have all been designed to handle the current daily sewage flows from existing students in the existing school building and its newly proposed modular classroom. (See Architect's Drawings)

3.1 Potable water:

The school potable water is provided by the Royston Water Service. The water service is metered and recorded every two months. The school's service and consumption records from 2016 to 2019 have been obtained. (See Appendix # 4) These records provide actual daily usage and average flow per student. The average consumption rate of a student using the greatest metered usage is 13.8L per student. That's following the highest (fall/winter) metered usage of the school in between 2016-2019. The late spring and summer months were not used as the school's playing fields are irrigated for community use.

4.0 Summary of System Performance:

The compliance inspection found all components of the existing and temporary systems to be in good working condition, and wastewater flows traveled through the system in a normal manner. Based on these observations, this system is operating in a normal and safe manner.

The current operations appear to be within the design parameters of the original construction drawings and existing system on the property. There are currently 267 student using approximately 15 lpd. The actual water meter records show a lower useage of only 13.8 lpd. Following SPM3 Table III-11 shows a daily average of 15 lpd to 30 lpd per student.

The current system has an VIHA Authorization of 22,500 lpd system design of the innovative protocol. The sewage disposal system that the school is currently using was designed for 300 students at 68 lpd under SSR Reg 411/86 for a total daily design flow (DDF) of 20,400 lpd. The 68 lpd per student was the design flow used under the regulation of the day.

The total Daily Design Flow using the SPM3 and actual water consumption will be 4800 lpd: 320 students @ 15 lpd each totals 4800 lpd.

Following Table II-22 SPM3, the soil Hydraulic loading rate for sandy loams is 80l/p/m2 Type 3 (L/Day/M2). Following section II-6.6 of SPM3, the existing system has a total of 122m of dispersal field length. Following the Standards of SPM3, the system's dispersal field is large enough to both safely and adequately disperse the treated effluent. The proposed DDF (4800 lpd) and existing approved system design only requires a total disperal field length of 100m.

When using the SPM3 design criteria and the site soils conditions, it appears the original system can treat the new daily sewage flows of the proposed 320 students.

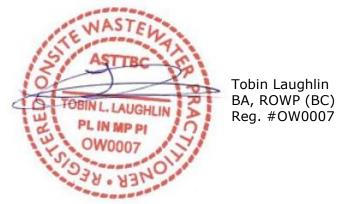
The new additional DDF of 750 lpd added to the current 4000 lpd the total system design flow of approximately 4800 lpd, is well under the sewage treatment plant's intended treatment design of 22,500 lpd. To ensure the system and its dispersal field function correctly, a conservative operation should follow the SPM3 system design criteria. When applying the SPM3 design criteria with proper operations and maintenance, the system will service the needs of the school's sewage demand.

The additional students will not contribute to a health hazard with the system being well maintained. Based on the as-built drawings, the components inspected at site, the system appears to be operating correctly and is not causing a health hazard. The intended use and function appears to be in compliance and meets the intent of the SSR.

The attached drawing shows the confirmed location of the system in relation to the existing and proposed unit and its design flow schematic.

It is understood that School District 71 pumps the septic tanks and pump chamber annually. It would be good operation standards to have the entire system inspected annually to ensure the system's function continues to operate with in the Sewage System Regulations. As there are no records on file with VIHA, the system's design and correct operation are unknown. Annual inspection would ensure correct operation and protect public health.

Should you have any other questions, please contact our office at any time.





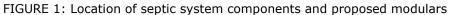
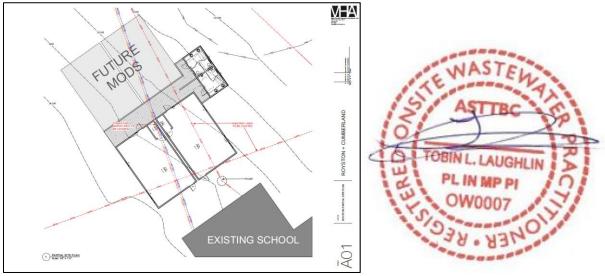


FIGURE 2:





Septic Systems and Treatment Plants / Service and Maintenance Inspections and Assessments / Permits and Design

FOR COMPLIANCE INSPECTION ONLY

Appendix 1: Statement of General Conditions

Scope of this Report

This review report satisfies only those objectives stated in the introduction. Tobin Laughlin (ROWP) has not conducted a *Site Investigation*, *Vertical Separation analysis, soils Assessment*, or Effluent sampling or testing.

Use of this Report

This Rock Creek Environmental (RCE) report pertains only to a specific project. If the project is modified, then our client will allow us to confirm that the report is still valid. We prepared this report only for the benefit of our Client and those agencies authorized by law to regulate our Client's activities.

No others may use any part of this report without our written consent. To understand the content of this report, the reader must refer to the entire, signed report. We cannot be responsible for the consequences of anyone using only a part of the report, or referring only to a draft report. This report reflects our best judgment based on information available at the time.

Any use of this report, or reliance on this report, by a third party is the responsibility of that third party. We accept no responsibility for damages, if any, suffered by a third party as a result of decisions made or actions taken based on this report.

Reliance on Provided Information

RCE has relied on the accuracy and completeness of information provided by its client and by other professionals. We are not responsible for any deficiency in this document that results from a deficiency in this information.

Site, Record Documents and Sewage Equipment Interpretations

Site and the sewage system equipment conditions always vary across a site and vary with time. Monitoring wells, inspection ports and the sewage system equipment maintenance logs show conditions only at the locations that can be inspected. The precision with which compliance reports show subsurface field and treatment equipment conditions depends on the recorded documents, access, maintenance records/logs, frequency and methods of sampling and testing, and the uniformity of subsurface conditions.

RCE based these descriptions on observations at the time of the study. Unless otherwise noted, we based the report's conclusions and recommendations on these observed conditions.

Changed Conditions

Conditions encountered by others at this site may differ significantly from what we encountered, either due to natural variability of equipment conditions, or as a result of other(s) activities. Our client will inform us about any such changes, and will give us an opportunity to review our recommendations. Recognizing changed soil, Design Flows and equipment conditions, requires experience. Therefore, during any construction or remediation, a qualified person should be employed to visit the site with sufficient frequency to observe whether conditions or system performance have changed significantly.

Risks and Liability

We recommend that our client engage RCE to review all design drawings and constructed works that are based on our conclusions and recommendations.

Standard of Care

We exercise a standard of care consistent with that level of skill and care ordinarily exercised by Authorized Persons & Professionals currently practicing under similar conditions.

APPENDIX 2

Sewerage System Standard Practice Manual Version 3

Volume II

Table II- 19. Minimum required horizontal separation distances

MINIMUM HORIZONTAL DISTANCE TO	FROM DISPERSAL	FROM WATERTIGHT TREATMENT OR PUMP TANK METRES	
	METRES		
Water sources and wells			
Surface source of drinking water	30	15	
Domestic water supply well ¹	30	30	
Domestic water supply well, high pumping rate ²	60	30	
Domestic water supply well, high pumping rate, in unconfined aquifer ²	90	30	
Irrigation well or open loop geothermal well	15	7.5	
Deep monitoring well or closed loop geothermal well ³	6	6	
Shallow monitoring well ⁴	3	0	
Drinking water lines and cisterns			
Drinking water suction line	30	15	
Drinking water suction line, sleeved ⁵	7.5	3	
Drinking water line, under pressure	3	3	
Drinking water line, under pressure, sleeved ^s	1	1	
Drinking water supply cistern, below ground	15	3	
Water bodies and surface breakout			
Permanent fresh water body ⁶	30	10	
Intermittent fresh water body ⁷	15	10	
Marine water body ⁸	15	7.5	
Break-out point or downslope drain ⁹	7.5	0	

Notes:

¹ For drinking water well, see the SSR s3.1 and Section II- 2.1.2.2 of this Manual for special considerations. Domestic water supply wells include excavated or dug wells.

² For definitions of "high pumping rate well" and "unconfined aquifer" see the glossary.

³ The horizontal separation to a deep monitoring well or to a closed loop geothermal well is based on a well with an annular seal that complies with the Ground Water Protection Regulation (GWPR). If the well does not comply with the GWPR, follow horizontal separation standards for drinking water wells.

⁴ The horizontal separation to a shallow monitoring well is based on a well which is shallower than 4.6 m and constructed with an annular seal that complies with the GWPR.

⁵ Sleeved water lines (suction or pressure) use continuous pipe sleeving within the normal standard HS to allow reduced HS, see Volume III for details.

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Sewerage System Standard Practice Manual Version 3

Volume III

III- 5.4.6 HORIZONTAL SEPARATIONS TO REDUCE RISK OF DAMAGE OR ENCROACHMENT

Horizontal separation is useful to reduce the risk of accidental system damage or encroachment on a neighboring property, and Table III- 16 provides guidance for these situations.

Table III- 16. Guidelines for minimum horizontal separation distances

DISTANCE TO	FROM DISPERSAL SYSTEM (METRES)	FROM LAGOON (METRES)	FROM WATERTIGHT SUBSURFACE TREATMENT TANK (METRES)
Property lines	3	*	1
Building or structure (where there is not a perimeter drain)	1	•	1
Dispersal system (including other dispersal system)	6	6	3
Buried utility services	1	1	1
Drinking water supply cistem, at or above ground	1	*	1

Notes:

- * For lagoon separations in these cases see Volume II, Table II- 20 (Page II-33)
- For swimming pools or lined ponds with no external subsurface drainage, use the horizontal setbacks for a building or structure for all systems except for BC zero discharge lagoons. Always provide access for maintenance work.
- If these guidelines are departed from, the AP should include in their rationale steps taken to mitigate risk
 of system damage or encroachment.
- Buried utility services include sewer, stormwater, electricity, gas, cable and telephone. For any work near buried services follow guidelines published by the utility.
- In freezing conditions it is important to consider setback to roads, driveways and other similar areas where:
- Soil may be compacted.
- Snow cover may be disturbed, compacted or removed.

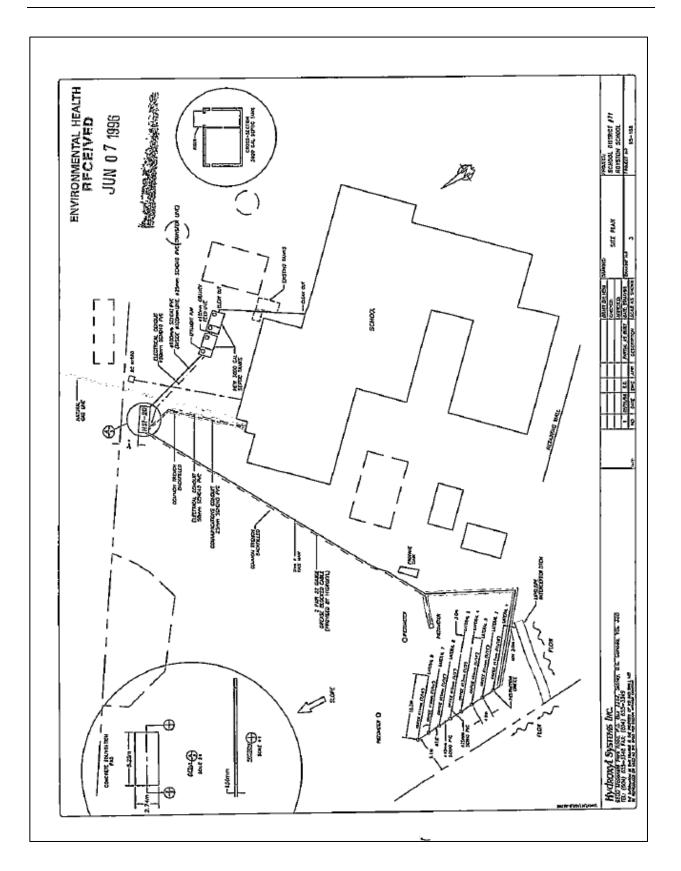
A separation of 3 m to these areas from system components may reduce risk of system freezing.

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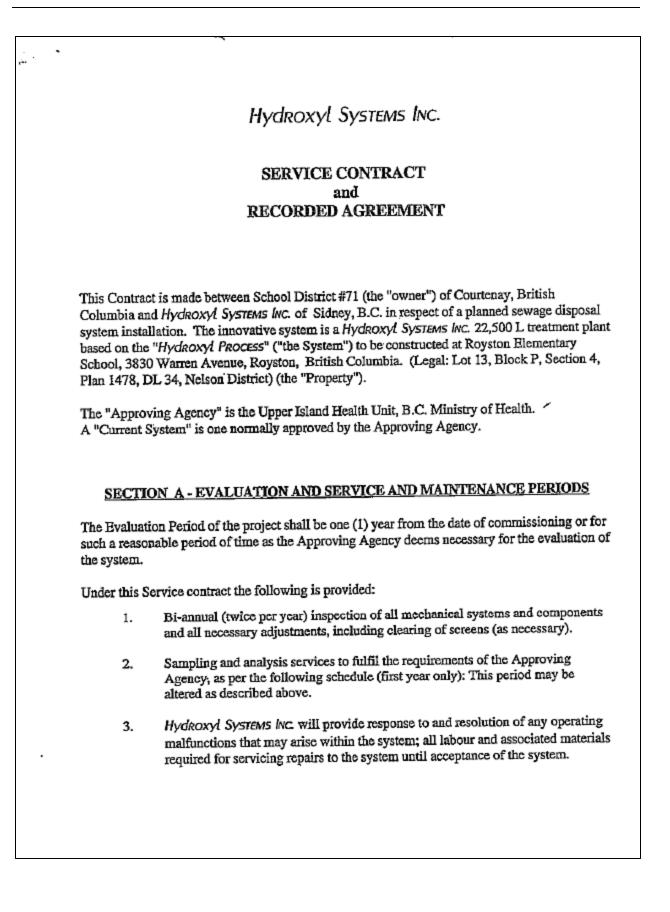
APPENDIX 3

	FOLIO NUMBER DA	TE OF APPLICATION (Y / M / D)			Construction	M Repa	ir 🗌	Alteration		
	771-09169.100	96 01 12		NEW	Construction					
WNER	NAME OF OWNER			44)		TELEPHONE NUMBER				
orrespondence	SCHOOL DISTRICT 71 COURTER			City		Pos	tal Code			
pe sent to		MBERLAND RO	AD	10.70	URTENAY		V9N 7	GS		
Downer APPLICANT NFORMATION Correspondence to be sent to Capplicant	NAME OF APPLICANT					TEL	EPHONE NUME	BER		
							334-	5500		
	Number and Street MAILING ACCRESS			Ci	N	Pos	ital Code			
т	LEGAL DESCRIPTION OF WHERE DISPO	SAL SYSTEM IS TO BE CONSTRU	UCTED							
ORMATION	LOT 13 PH	IN 1478 SEC	4 DL 34	NE	ilson Lb	(BLOCK	P)			
-	entral entering internal designed									
	STREET ADDRESS / GENERAL LOCATION	3830 WAR	REN AVE	• 1	ZONSTON					
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ORMATION		DUPLEX	N/A		TYes PNo			FUTUREY		
	TOTHER (specify):	N. SCHOOL	TOTAL LIVING	REA	-	LOT SIZE:	1 10 110			
	ESTIMATED DAILY SEWAGE FLOW	5000 IGPD	Z	556	MZ	5	.4 AC	-		
STEM	TYPE OF SEWAGE DISPOSAL SYSTEM:			SEPT	IC TANK MANUFACTU			LIQUID VOLUME OF TANK		
ORMATION				HYDROXYL SYSTEMS INC.						
	ST ALTERNATE (E.G. T PRIVY TILA			MATE	FIAL OF SEPTIC TANK			1		
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	IF PACKAGE TREATMENT PLANT IS PRO	1	TREATMENT	SEV	VAGE PUMP :	FIELD DOS	the second s	PED PER CYCLE		
	MAKE HYDROXYL SY	CAPACITY	NES YES		AS PER PLANT DISCHARGE SPE					
	NODEL HSI-23		4950 16PD 22500 LPD	D	NO	ASPER	Pariet Bi	scorringe are		
TERNATE	PRESSURE DISTRIBUTION PROPOSED	LAGOON SIZE		DEPTH OF CLAY SOIL		GARBURATOR				
ORMATION	OF YES O NO					C YES		NO		
-	SOIL DESCRIPTION DEPTH OF SOIL O over 1.2 m (41	-			, due to ⊡ rock nder 1.2m (4 ft.) t			om surface.		
ORMATION	DEPTH TO WATER YABLE: O OVER 1	PERCTESTS								
ORMATION	PERC TESTS		14.1.1.1	1.00	SLOWEST RATE FROM test hole #1 24 min./2.5 cm (1 inch) test hole #2 28 min./2.5 cm (1 inch)					
ORMATION	PERC TESTS SLOWEST RATE FROM 1851 hole 1		2			<u>8</u> min./ 2.3	i can (1 mea)			
ORMATION	PERC TESTS SLOWEST RATE FROM 1951 hole 4 AVERAGE OF SLOWEST RATE FROM EAC		n (1 inch) min./ 2.5 cm			s min./ 2.3	i chi (1 mort			
ORMATION	PERC TESTS SLOWEST RATE FROM 185t hole a AVERAGE OF SLOWEST RATE FROM EAC WATER INFORMATION		min./ 2.5 cm	(tinc	h)	<u>s</u> min./ 2.3	, can (1 man)			
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ORMATION STRICTIVE VENANTS PLICANT INATURE	PERC TESTS SLOWEST RATE FROM IBST hole 4 AVERAGE OF SLOWEST RATE FROM EAC WATER INFORMATION SOURCES OF DOMESTIC WATER: DISTANCES OF PROPOSED DISPOSAL FI/A	ROY STON IMPI	min./ 2.5 cm 2006/MENT (+ 100' sour wells wells wells of myknowledge: Em S	rce of	h)	ike		AL HEALTH		

387/94 AUTHORIZATION TO OPERATE Ministry of H) and Province of British Columbia Ministry Responsible for Seniors A SEWAGE DISPOSAL SYSTEM NAME OF CONTRACTOR DATE OF APPLICATION (Y/M/D) NAME OF OWNER FOLIO NUMBER Hydrox Systems Inc. 96/01/12 School District 71 Courtever 771-09169.100 LEGAL DESCRIPTION OF LOT STREET ADDRESS / GENERAL LOCATIO 3930 Whenen Ave, Royston, BC Lot 13 Man 1478 Sec + DL 3+ INSTALLED AS PER REGULATIONS SIGNATURE OF OWNER / APPLICANT Nelson LD (Block P) 🕱 Yes D No AS BUILT DIAGRAM : to be completed by the contractor or applicant Scale 1 Box + n file ____ 419-28 196,699 A sponse sponted ind LOWING MOUTH Carthole, (m-b)d # 120er ingening press 010-7 RECEIVE (Carter of the second NUL 101 5.2 and the design 07 \$110.2 1996 HEALTH 0 R PARK The Ministry of Health does not guarantee the useable life of the sewage disposal system. The life of the system is affected by the use and maintenance it receives. Pump out the septic tank every 2-3 years. For servicing of package treatment plants, consult your local service agent. For service guarantees, if the If the system is not authorized for backfilling and if corrections are required, a re-inspection fee of \$100 must be paid for each time the Public Health inspector checks to see that the faults have been corrected. DATE BACKFILL / USE SUBJECT TO THE EQLLOWING CONDITIONS cond AUTHORIZED m of 5 0 ter rpo mu 5 tac SIGNATURE PUBLIC HEALTH INSPECTOR / EHO: FOR PUBLIC HEALTH INSPECTOR / EHO USE ONLY APPROVED REJECTED NOT APPLICABLE APPROVED, REJECTED NOT APPLICABLE ø 0 0 curtain drain O septic tank interceptor drains 0 P package treatment prem other (e.g.lagcon,holding tank) sperify package treatment plant pump. D Hydroxy ۵ D drain rock Ò set back distances D 0 0 distribution box 101 п siphon m HLTH 136 Rev. 94/09 WH/TE COPY - FILE YELLOW COPY - TO OWNER PINK COPY - TO BUILDING AUTHORITY



	Environmental Health Upper Island Health Unit	Building Departmant REFERRAL Bui ung Inspections Regional District of Comox-Strathcona
OFFICE	941B England Avenue	P.O. Box 3370 RECEIVED
0	Courtenay, B.C. V9N 2IN7	Courtenay, B.C. ADD 9 F 1000
- Р	hone: 334-1172 Fax: 334-1439	W Phone: 334-6000
EXPECTE	D OCCUPANCY INCREASE	G EXTENSION CHANGE IN USE OTHER
LEGAL DESCRIPTION (as applicable)	PCL LOT DL BLK	SEC. TWP. LAND DISTRICT PLAN NO. 44 MRL Saw 1478
HOUSE NUMBER		PISTRICT DD GF 14687
OT DIMENSION	112 X 12/m 5.4 ACRES	RESTRICTIVE COVENANTS AFFECTING THE PROPOSAL:
SSESSMENT	3-1 010 -1	OWNER'S NAME
XISTING DW	ELLING DETAILS:	ADDRESS:
otal Floor Area: . otal Bedrooms: _	Year Constructed:	Agent S NAME:
	AGE DISPOSAL SYSTEM:	ADDRESS:
te of Last Alteration		MUNICIPALITY: POSTAL CODE: PHONE:
	L (nature and extent of building modificatio	ons): Total Bedrooms Finished Floor Area 960
INSTALL		PERMINICIPIT) NO PRUMBING (DRY TYPH)
numbe	r of students	(1944) an internal (may 1944)
LOT PLAN:	(the following information must be transferred on a copy	of the survey certificate plan or representative scale diagramic
Lot Dimensions		
E NORTH ATTOM	Proposed area of building expan-	rviced buildings. sion (show dotted lines)
] Roads/Intersect	ions Proposed area of building expan Existing and proposed drinking	sion (show dotted lines) water sources and waterlines.
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-2-

System of 22,500 Litres or Less

Time from Startup	Parameters	by
1 month	Field: Visual Inspection	Hydroxyl Systems Inc.
2 months	Effluent: BOD5, TSS, Fecal Coliform Field: Visual Inspection	Testing Lab Hydroxyl Systems Inc.
3 months	Field: Visual Inspection	Hydroxyl Systems Inc.
4 months	Effluent: BOD5, TSS, Fecal Coliform Field: Visual Inspection	Testing Lab Hydroxyl Systems Inc
5 months	Field: Visual Inspection	Hydroxyl Systems Inc
6 months	Effluent: BOD5, TSS, Fecal Coliform Field: Visual Inspection	Testing Lab Hydroxyl Systems Inc
8 months	Effluent: BOD5, TSS, Fecal Coliform Field: Visual Inspection	Testing Lab Hydroxyl Systems Inc
10 months	Field: Visual Inspection	Hydroxyl Systems Inc
12 months	Effluent: BOD5, TSS, Fecal Coliform Field: Visual Inspection	Testing Lab Hydroxyl Systems Inc

At least once per winter, the drainfield will be checked after a major rainfall event.

To guarantee impartiality, all laboratory testing of effluent samples will be accomplished by an independent test laboratory. Results will be forwarded to both the Approving Agency and HSL.

SECTION B - OWNER RESPONSIBILIFIES

The Owner agrees to:

- allow access to the Property by the Service Agent and the Approving Agency for the purposes of routine inspection, sampling, monitoring or necessary repair or enforcement action;
- not remove or damage the System, place any materials on the System, fail to supply electrical energy to the System or take any other action which hinders the operation of the system;
- notify prospective purchasers or other parties of this agreement before sale or transfer of ownership.

-3-

SECTION C - SERVICE AGENT RESPONSIBILITIES

In return for the Owner's payment of the Service and Monitoring Fee, the Service Agent agrees to provide servicing (the Service) for as long as the system is used in waste water disposal.

Two Service Options are available for the system for the period following acceptance by the Approving Agency:

Option A: Routine Service and Maintenance.

Under this Service Contract, the following is provided:

Bi-annual (twice per year) inspection of all mechanical and system components and all necessary adjustments, including clearing of screens (as necessary).

Option B: Comprehensive Service, Maintenance and Repair

In addition to the services provided under Option A, Hydroxyl Systems Inc. will provide:

- response to, and resolution of, any operating malfunctions that may arise with the system;
- all labour and associated materials required for servicing and repairs to the system.

General

The above servicing and monitoring program is intended to verify performance and comply with the requirements of the Approving Agency. Additional sampling and analysis for research purposes may be conducted by the Service Agent with the permission of the Owner.

Hydroxyl Systems INC. commits that the performance of the System under normal use conditions will meet the following standards for effluent quality: not more than 10mg/L 5-day Biochemical Oxygen Demand (BOD5); not more than 5mg/L Total Suspended Solids (TSS), not more than 100 MPN fecal colliforms per 100 mI and not more than 25mg/L nitrate nitrogen. It is understood that occasional exceedences of these targets can occur.

Hydroxyl Systems INC.'s obligation to guarantee performance under this contract are deemed to be suspended if:

 the Owner fails to observe good rules of practice in the disposal of water and substances to the System, as set out in the Owner's Manual to be provided to the Owner by the Service Agent;

APPENDIX 4 ROYSTON WATER CONSUMPTION HISTORY							
Site Descri	ption	Reading Start	Reading End	Consumption	Cost		
3830 Warren Ave, C	Comox, BC	July 1, 2016	July 31, 2016	1,220.00	\$1,522.10		
3830 Warren Ave, C	Comox, BC	August 1, 2016	August 31, 2016	1,200.00	\$1,497.30		
3830 Warren Ave, C	Co mox, BC	September 1, 2016	September 30, 2016	690.00	\$864.90		
3830 Warren Ave, C	Comox, BC	October 1, 2016	October 31, 2016	75.00	\$102.30		
3830 Warren Ave, C	Comox, BC	November 1, 2016	November 30, 2016	50.00	\$71.30		
3830 Warren Ave, C	Comox, BC	December 1, 2016	December 31, 2016	20.00	\$41.40		
3830 Warren Ave, C	Comox, BC	January 1, 2017	January 31, 2017	80.00	\$108.50		
3830 Warren Ave, C	Comox, BC	February 1, 2017	February 28, 2017	120.00	\$158.10		
3830 Warren Ave, C	Comox, BC	March 1, 2017	March 31, 2017	120.00	\$158.10		
3830 Warren Ave, C	Comox, BC	April 1, 2017	April 30, 2017	110.00	\$145.70		
3830 Warren Ave, C	Comox, BC	May 1, 2017	May 31, 2017	575.00	\$722.30		
3830 Warren Ave, C	Comox, BC	June 1, 2017	June 30, 2017	1,910.00	\$2,377.70		
3830 Warren Ave, C	Comox, BC	July 1, 2017	July 31, 2017	1,265.00	\$1,577.90		
3830 Warren Ave, C	Comox, BC	August 1, 2017	August 31, 2017	2,065.00	\$2,569.90		
3830 Warren Ave, C	Comox, BC	September 1, 2017	September 30, 2017	265.00	\$337.90		
3830 Warren Ave, O	Comox, BC	October 1, 2017	October 31, 2017	70.00	\$96.10		
3830 Warren Ave, C	Comox, BC	November 1, 2017	November 30, 2017	50.00	\$71.30		
3830 Warren Ave, C	Comox, BC	December 1, 2017	December 31, 2017	50.00	\$71.30		
3830 Warren Ave, C	Comox, BC	January 1, 2018	January 31, 2018	75.00	\$102.30		
3830 Warren Ave, C	Comox, BC	February 1, 2018	February 28, 2018	55.00	\$77.50		
3830 Warren Ave, C	Comox, BC	March 1, 2018	March 31, 2018	80.00	\$108.50		
3830 Warren Ave, C	Comox, BC	April 1, 2018	April 30, 2018	105.00	\$139.50		
3830 Warren Ave, C	Comox, BC	May 1, 2018	May 31, 2018	1,205.00	\$1,503.50		
3830 Warren Ave, C	Comox, BC	June 1, 2018	June 30, 2018	1,740.00	\$2,166.90		
3830 Warren Ave, C	Comox, BC	July 1, 2018	July 31, 2018	1,125.00	\$1,404.30		
3830 Warren Ave, O	Comox, BC	August 1, 2018	August 31, 2018	695.00	\$871.10		
3830 Warren Ave, O	Comox, BC	September 1, 2018	September 30, 2018	350.00	\$443.30		
3830 Warren Ave, O	Comox, BC	October 1, 2018	October 31, 2018	40.00	\$61.00		
3830 Warren Ave, O	Comox, BC	November 1, 2018	November 30, 2018	40.00	\$61.00		
3830 Warren Ave, O	Comox, BC	December 1, 2018	December 31, 2018	70.00	\$96.10		
3830 Warren Ave, O	Comox, BC	January 1, 2019	January 31, 2019	40.00	\$61.00		
3830 Warren Ave, O	Comox, BC	February 1, 2019	February 28, 2019	50.00	\$71.30		
3830 Warren Ave, O	Comox, BC	March 1, 2019	March 31, 2019	40.00	\$61.00		
3830 Warren Ave, O	Comox, BC	April 1, 2019	April 30, 2019	40.00	\$61.00		
3830 Warren Ave, O	Comox, BC	May 1, 2019	May 31, 2019	980.00	\$1,224.50		
3830 Warren Ave, O	Comox, BC	June 1, 2019	June 30, 2019	890.00	\$1,112.90		
3830 Warren Ave, O	Comox, BC	July 1, 2019	July 31, 2019	800.00	\$1,001.30		
3830 Warren Ave, O	Comox, BC	August 1, 2019	August 31, 2019	150.00	\$195.30		
3830 Warren Ave, O	Comox, BC	September 1, 2019	October 31, 2019	105.00	\$139.50		
3830 Warren Ave, O	Comox, BC	November 1, 2019	November 30, 2019	35.00	\$55.85		
3830 Warren Ave, O	Comox, BC	December 1, 2019	December 31, 2019	40.00	\$116.84		
3830 Warren Ave, C	Comox, BC	January 1, 2020	February 29, 2020	265.00	\$398.90		

APPENDIX 4 ROYSTON WATER CONSUMPTION HISTORY

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